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# 46088

STATE OF NORTH CAROLINA

DEPARTMENT OF TRANSPORTATION **DIVISION OF HIGHWAYS** GEOTECHNICAL ENGINEERING UNIT

#### **STRUCTURE** SUBSURFACE INVESTIGATION

COUNTY STANLY

PROJECT DESCRIPTION REPLACE BRIDGE NO. 44 OVER LONG CREEK ON SR 1435 (POPLIN ROAD)

STATE	STATE PROJECT REPERENCE NO.	SHEET NO.	TOTAL SHEETS
N.C.	B-5373	1	12

#### **CAUTION NOTICE**

THE SUBSURFACE INFORMATION AND THE SUBSURFACE INVESTIGATION ON WHICH IT IS BASED WERE MADE FOR THE PURPOSE OF STUDY, PLANNING AND DESIGN, AND NOT FOR CONSTRUCTION OR PAY PURPOSES. THE VARIOUS FIELD BORING LOGS, ROCK CORES AND SOIL TEST DATA AVAILABLE MAY BE REVIEWED OR INSPECTED IN RALEIGH BY CONTACTING THE N. C. DEPARTMENT OF TRANSPORTATION, GEOTECHNICAL ENGINEERING UNIT AT 1999 707-6805. THE SUBSURFACE PLANS AND REPORTS, FIELD BORING LOGS, ROCK CORES AND SOIL TEST DATA ARE NOT PART OF THE CONTRACT.

GENERAL SOIL AND ROCK STRATA DESCRIPTIONS AND INDICATED BOUNDARIES ARE BASED ON A GEOTECHNICAL INTERPRETATION OF ALL AVAILABLE SUBSURFACE DATA AND MAY NOT NECESSARILY REFLECT THE ACTUAL SUBSURFACE CONDITIONS BETWEEN BORINGS OR BETWEEN SAMPLED STRATA WITHIN THE BOREHOLE. THE LABORATORY SAMPLE DATA AND THE IN SITU (IN-PLACE) TEST DATA CAN BE RELIED ON ONLY TO THE DEGREE OF RELIABILITY INHERENT IN THE STANDARD TEST METHOD. THE OBSERVED WATER LEVELS OR SOIL MOISTURE CONDITIONS INDICATED IN THE SUBSURFACE INVESTIGATION. THESE WATER LEVELS OR SOIL MOISTURE CONDITIONS MAY VARY CONSIDERABLY WITH TIME ACCORDING TO CLIMATIC CONDITIONS MEDICATED DESCRIPTIONS AND ASSECTIONS OF THE INVESTIGATION. THE STATEM LEVELS OR SOIL MOISTURE CONDITIONS MAY VARY CONSIDERABLY WITH TIME ACCORDING TO CLIMATIC CONDITIONS MAY VARY CONSIDERABLY WITH TIME ACCORDING TO CLIMATIC CONDITIONS INCLUDING TEMPERATURES, PRECIPITATION AND WIND, AS WELL AS OTHER NON-CLIMATIC FACTORS.

THE BIDDER OR CONTRACTOR IS CAUTIONED THAT DETAILS SHOWN ON THE SUBSURFACE PLANS ARE PRELIMINARY ONLY AND IN MANY CASES THE FINAL DESIGN DETAILS ARE DIFFERENT, FOR BIDDING AND CONSTRUCTION PURPOSES, REFER TO THE CONSTRUCTION PLANS AND DOCUMENTS FOR FINAL DESIGN INFORMATION ON THIS PROJECT. THE DEPARTMENT DOES NOT WARRANT OR GUARANTEE THE SUFFICIENCY OR ACCURACY OF THE INVESTIGATION MADE, NOR THE INTERPRETATIONS MADE, OR OPINION OF THE DEPARTMENT AS TO THE TYPE OF MATERIALS AND CONDITIONS TO BE ENCOUNTERED. THE BIDDER OR CONTRACTOR IS CAUTIONED TO MAKE SUCH INDEPENDENT SUBSURFACE INVESTIGATIONS AS HE DEEMS NECESSARY TO SATISTY HIMSELF AS TO CONDITIONS TO BE ENCOUNTERED ON THE PROJECT. THE CONTRACTOR SHALL HAVE NO CLAIM FOR ADDITIONAL COMPENSATION OR FOR AN EXTENSION OF TIME FOR ANY REASON RESULTING FROM THE ACTUAL CONDITIONS ENCOUNTERED AT THE SITE DIFFERING FROM THOSE INDICATED IN THE SUBSURFACE INFORMATION.

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  OR CONTRACT FOR THE PROJECT.
  BY HAVING REQUESTED THIS INFORMATION, THE CONTRACTOR SPECIFICALLY WAIVES ANY CLAIMS
  FOR INCREASED COMPENSATION OR EXTENSION OF TIME BASED ON DIFFERENCES BETWEEN THE
  CONDITIONS INDICATED HEREIN AND THE ACTUAL CONDITIONS AT THE PROJECT SITE.

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CHECKED BY J.E. BEVERI	
SUBMITTED BY K.B. MILL	ER
DATE FEBRUARY 2017	

PERSONNEL



**UNLESS ALL SIGNATURES COMPLETED** 

PROJECT REFERENCE NO. SHEET NO. 2

## NORTH CAROLINA DEPARTMENT OF TRANSPORTATION DIVISION OF HIGHWAYS GEOTECHNICAL ENGINEERING UNIT

#### SUBSURFACE INVESTIGATION

SOIL AND ROCK LEGEND, TERMS, SYMBOLS, AND ABBREVIATIONS

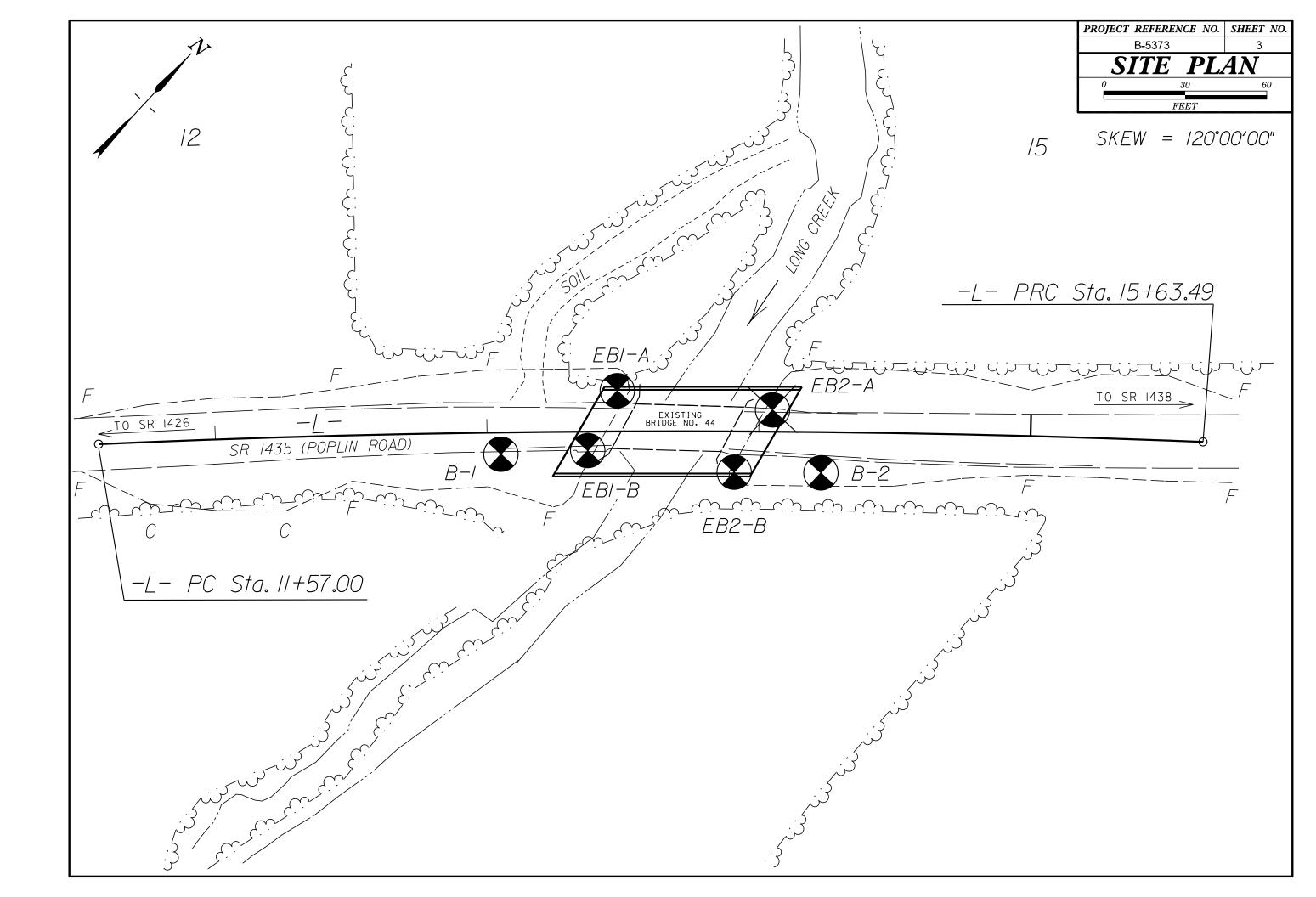
SOIL DESCRIPTION	GRADATION	ROCK DESCRIPTION	TERMS AND DEFINITIONS
SOIL IS CONSIDERED UNCONSOLIDATED, SEMI-CONSOLIDATED, OR WEATHERED EARTH MATERIALS THAT CAN BE PENETRATED WITH A CONTINUOUS FLICHT POWER AUGER AND YIELD LESS THAN 100 BLOWS PER FOOT ACCORDING TO THE STANDARD PENETRATION TEST (AASHTO T 206, ASTM DI586). SOIL CLASSIFICATION IS BASED ON THE AASHTO SYSTEM. BASIC DESCRIPTIONS GENERALLY INCLUDE THE FOLLOWING: CONSISTENCY, COLOR, TEXTURE, MOISTURE, AASHTO CLASSIFICATION, AND OTHER PERTINENT FACTORS SUCH AS MINERALOGICAL COMPOSITION, ANDULARITY, STRUCTURE, PLASTICITY, ETC. FOR EXAMPLE, VERY STRYF, FARY, SUTY CLAY, WOIST WITH WITERBEDDED FINE SAMD QUAPER, HIGHLY SATIC, A-7-6	WELL GRADED - INDICATES A GOOD REPRESENTATION OF PARTICLE SIZES FROM FINE TO COARSE.  UNIFORMLY GRADED - INDICATES THAT SOIL PARTICLES ARE ALL APPROXIMATELY THE SAME SIZE.  GAP-GRADED - INDICATES A MIXTURE OF UNIFORM PARTICLE SIZES OF TWO OR MORE SIZES.  ANGULARITY OF GRAINS  THE ANGULARITY OR ROUNDNESS OF SOIL GRAINS IS DESIGNATED BY THE TERMS:	HARD ROCK IS NON-COASTAL PLAIN MATERIAL THAT WOULD YIELD SPT REFUSAL IF TESTED, AN INFERRED ROCK LINE INDICATES THE LEVEL AT WHICH NON-COASTAL PLAIN MATERIAL WOUL YIELD SPT REFUSAL, SPT REFUSAL IS PENETRATION BY A SPLIT SPOON SAMPLER EQUAL TO OR LESS THAN 0.1 FOOT PER 60 BLOWS IN NON-COASTAL PLAIN MATERIAL. THE TRANSITION BETWEEN SOIL AND ROCK IS OFTEN REPRESENTED BY A ZONE OF WEATHERED ROCK.  ROCK MATERIALS ARE TYPICALLY DIVIDED AS FOLLOWS:	ALLUVIUM (ALLUV.) - SOILS THAT HAVE BEEN TRANSPORTED BY WATER.  AQUIFER - A WATER BEARING FORMATION OR STRATA,  ARENACEOUS - APPLIED TO ROCKS THAT HAVE BEEN DERIVED FROM SAND OR THAT CONTAIN SAND,  ARGILLACEOUS - APPLIED TO ALL ROCKS OR SUBSTANCES COMPOSED OF CLAY MINERALS, OR HAVING  A NOTABLE PROPORTION OF CLAY IN THEIR COMPOSITION, SUCH AS SHALE, SLATE, ETC.
SOIL LEGEND AND AASHTO CLASSIFICATION   GENERAL   GRANULAR MATERIALS   SILT-CLAY MATERIALS   ORGANIC MATERIALS   CLASS. (≤ 35% PASSING *200)   (> 35% PASSING *200)   ORGANIC MATERIALS	ANGULAR, SUBANGULAR, SUBROUNDED, OR ROUNDED.  MINERAL OGICAL COMPOSITION  MINERAL NAMES SUCH AS QUARTZ, FELDSPAR, MICA, TALC, KAQLIN, ETC.  ARE USED IN DESCRIPTIONS WHEN THEY ARE CONSIDERED OF SIGNIFICANCE.	WEATHERED ROCK (WR)  NON-COASTAL PLAIN MATERIAL THAT WOULD YIELD SPT N VALUES > 100 BLOWS PER FOOT IF TESTED.  CRYSTALLINE ROCK (CR)  FINE TO COARSE GRAIN IGNEOUS AND METAMORPHIC ROCK THAT WOULD YIELD SPT REFUSAL IF TESTED. ROCK TYPE INCLUDES GRANITE, GOKEISS, GABBRO, SCHIST, ETC.	ARTESIAN - GROUND WATER THAT IS UNDER SUFFICIENT PRESSURE TO RISE ABOVE THE LEVEL AT WHICH IT IS ENCOUNTERED, BUT WHICH DOES NOT NECESSARILY RISE TO OR ABOVE THE GROUND SURFACE.
CROUP A-1 A-3 A-2 A-4 A-5 A-6 A-7 A-1, A-2 A-4, A-5 CA-6 A-7 A-1, A-2 A-4, A-5 CA-6 A-7 A-1, A-2 A-4, A-5 CA-6 A-7 CA-6	COMPRESSIBLE LL < 31  MODERATELY COMPRESSIBLE LL = 31 - 50	NON-CRYSTALLINE ROCK (NCR)  FINE TO COARSE GRAIN METAMORPHIC AND NON-COASTAL PLAIN SEDIMENTARY ROCK THAT WOULD YELLD SPT REFUSAL IF TESTED. ROCK (NCR)  COASTAL PLAIN  COASTAL PLAIN SEDIMENTS CEMENTED INTO ROCK, BUT MAY NOT YIELD	CALCAREOUS (CALC.) - SOILS THAT CONTAIN APPRECIABLE AMOUNTS OF CALCIUM CARBONATE.  COLLUVIUM - ROCK FRAGMENTS MIXED WITH SOIL DEPOSITED BY GRAVITY ON SLOPE OR AT BOTTOM OF SLOPE.  CORE RECOVERY (REC.) - TOTAL LENGTH OF ALL MATERIAL RECOVERED IN THE CORE BARREL DIVIDED
7. PASSING 18 50 MX GRANULAR SILT- CLAY MUCK.	HIGHLY COMPRESSIBLE LL > 50  PERCENTAGE OF MATERIAL	SEDIMENTARY ROCK SPT REFUSAL, ROCK TYPE INCLUDES LIMESTONE, SANDSTONE, CEMENTED (CP) SHELL BEDS, ETC. WEATHERING	BY TOTAL LENGTH OF CORE RUN AND EXPRESSED AS A PERCENTAGE.  DIKE - A TABULAR BODY OF IGNEOUS ROCK THAT CUTS ACROSS THE STRUCTURE OF ADJACENT
*48 36 MX 56 MX 55 MN 1	ORGANIC MATERIAL GRANULAR SILT - CLAY OTHER MATERIAL	FRESH ROCK FRESH, CRYSTALS BRIGHT, FEW JOINTS MAY SHOW SLIGHT STAINING, ROCK RINGS UNDER	ROCKS OR CUTS MASSIVE ROCK.  DIP - THE ANGLE AT WHICH A STRATUM OR ANY PLANAR FEATURE IS INCLINED FROM THE
MATERIAL PASSING *40  LL 48 MX 41 MN 48 MX 41 MN 48 MX 41 MN 48 MX 41 MN 50ILS WITH LTTLE OR PI 6 MX NP 18 MX 18 MX 11 MN 11 MN 18 MX 18 MX 11 MN 11 MN LTTLE OR HIGHLY	TRACE OF ORGANIC MATTER 2 - 3% 3 - 5% TRACE 1 - 10% LITTLE ORGANIC MATTER 3 - 5% 5 - 12% LITTLE 10 - 20% MODERATELY ORGANIC 5 - 10% 12 - 20% SOME 20 - 35% HIGHLY ORGANIC > 10% > 20% HIGHLY 35% AND ABOVE	HAMMER IF CRYSTALLINE.  VERY SLIGHT (V SLI,)  CRYSTALS ON A BROKEN SPECIMEN FACE SHINE BRIGHTLY. ROCK RINGS UNDER HAMMER BLOWS IF OF A CRYSTALLINE NATURE.	HORIZONTAL.  DIP DIRECTION (DIP AZIMUTH) - THE DIRECTION OR BEARING OF THE HORIZONTAL TRACE OF THE LINE OF DIP, MEASURED CLOCKWISE FROM NORTH.
CROUP INDEX 0 0 0 4 MX 8 MX 12 MX 16 MX NO MX AMOUNTS OF SOILS  USUAL TYPES STONE FRAGS. FINE SILTY OR CLAYEY SILTY CLAYEY MATTER	GROUND WATER  WATER LEVEL IN BORE HOLE IMMEDIATELY AFTER DRILLING	SLIGHT ROCK GENERALLY FRESH, JOINTS STAINED AND DISCOLORATION EXTENDS INTO ROCK UP TO (SLI.) I INCH. OPEN JOINTS MAY CONTAIN CLAY. IN GRANITOID ROCKS SOME OCCASIONAL FELDSPAR	FAULT - A FRACTURE OR FRACTURE ZONE ALONG WHICH THERE HAS BEEN DISPLACEMENT OF THE SIDES RELATIVE TO DNE ANOTHER PARALLEL TO THE FRACTURE.
OF MAJOR GRAVEL, AND SAND GRAVEL AND SAND SOILS SOILS  GEN BOTHER	STATIC WATER LEVEL AFTER 24 HOURS  VPW PERCHED WATER, SATURATED ZONE, OR WATER BEARING STRATA	CRYSTALS ARE DULL AND DISCOLORED. CRYSTALLINE ROCKS RING UNDER HAMMER BLOWS.  MODERATE  (MOD.) SIGNIFICANT PORTIONS OF ROCK SHOW DISCOLORATION AND WEATHERING EFFECTS. IN  GRANITOID ROCKS, MOST FELDSPARS ARE DULL AND DISCOLORED, SOME SHOW CLAY. ROCK HAS	FISSILE - A PROPERTY OF SPLITTING ALONG CLOSELY SPACED PARALLEL PLANES.  FLOAT - ROCK FRAGMENTS ON SURFACE NEAR THEIR ORIGINAL POSITION AND DISLODGED FROM PARENT MATERIAL.
AS SUBGRADE	SPRING OR SEEP	DULL SOUND UNDER HAMMER BLOWS AND SHOWS SIGNIFICANT LOSS OF STRENGTH AS COMPARED WITH FRESH ROCK.  MODERATELY ALL ROCK EXCEPT QUARTZ DISCOLORED OR STAINED. IN GRANITOID ROCKS, ALL FELDSPARS DULL AND DISCOLORED AND A MADDITY CHAPTER OF THE STAINED AND CONTROL OF THE STAINED.	FLOOD PLAIN (FP) - LAND BORDERING A STREAM, BUILT OF SEDIMENTS DEPOSITED BY THE STREAM.  FORMATION (FM.) - A MAPPABLE GEOLOGIC UNIT THAT CAN BE RECOGNIZED AND TRACED IN THE FIELD.
CONSISTENCY OR DENSENESS  PRIMARY SOIL TYPE COMPACTNESS OR CONSISTENCY PRIMARY SOIL TYPE COMPACTNESS OR CONSISTENCY PRIMARY SOIL TYPE COMPACTNESS OR CONSISTENCY (N-YALUE)  RANGE OF STANDARD COMPRESSIVE STRENGTH (TONS/FT²)	MISCELLANEOUS SYMBOLS  ROADWAY EMBANKMENT (RE)  PO FROCK STRUCTURES  STRUCTURES	SEVERE AND DISCOLORED AND A MAJORITY SHOW KAOLINIZATION, ROCK SHOWS SEVERE LOSS OF STRENGTH MOD, SEV.)  AND CAN BE EXCAVATED WITH A GEOLOGIST'S PICK, ROCK GIVES 'CLUNK' SOUND WHEN STRUCK,  IF TESTED, WOULD YIELD SPT REFUSAL  SEVERE ALL ROCK EXCEPT QUARTZ DISCOLORED OR STAINED, ROCK FABRIC CLEAR AND EVIDENT BUT REDUCED IN STRENGTH TO STRONG SOIL. IN GRANITOID ROCKS ALL FELDSPARS ARE KAOLINIZED	<u>JOINT</u> - FRACTURE IN ROCK ALONG WHICH NO APPRECIABLE MOVEMENT HAS OCCURRED. <u>LEDGE</u> - A SHELF-LIKE RIDGE OR PROJECTION OF ROCK WHOSE THICKNESS IS SMALL COMPARED TO ITS LATERAL EXTENT.
GENERALLY VERY LOOSE ( 4   COOSE ( 5   COO	SOIL SYMBOL  SOIL SYMBOL  STOPP ONT TEST BORING  SLOPE INDICATOR INSTALLATION INSTALLATION COME PENETROMETER THAN ROADWAY EMBANKMENT  AUGER BORING  COME PENETROMETER TEST	TO SOME EXTENT. SOME FRAGMENTS OF STRONG ROCK USUALLY REMAIN.  IF TESTED, WOULD YIELD SYT N VALUES > 100 BFF  VERY  ALL ROCK EXCEPT QUARTZ DISCOLORED OR STAINED. ROCK FABRIC ELEMENTS ARE DISCERNIBLE SEVERE  BUT MASS IS EFFECTIVELY REQUEED TO SOIL STATUS, WITH ONLY FRAGMENTS OF STRONG ROCK	LENS - A BODY OF SOIL OR ROCK THAT THINS OUT IN ONE OR MORE DIRECTIONS.  MOTTLED (MOT.) - IRREGULARLY MARKED WITH SPOTS OF DIFFERENT COLORS, MOTTLING IN SOILS USUALLY INDICATES POOR AERATION AND LACK OF GOOD DRAINAGE.  PERCHED WATER - WATER MAINTAINED ABOVE THE NORMAL GROUND WATER LEVEL BY THE PRESENCE
VERY SOFT   < 2   < 0.25	INFERRED SOIL BOUNDARY  ———————————————————————————————————	(V SEV.)  REMAINING. SAPROLITE IS AN EXAMPLE OF ROCK WEATHERED TO A DEGREE THAT ONLY MINOR VESTICES OF ORIGINAL ROCK FABRIC REMAIN. IF TESTED, WOULD YIELD SPT N VALUES \(\) 100 BPF  COMPLETE  ROCK REDUCED TO SOIL. ROCK FABRIC NOT DISCERNIBLE, OR DISCERNIBLE ONLY IN SMALL AND SCATTERED CONCENTRATIONS. QUARTZ MAY BE PRESENT AS DIKES OR STRINGERS. SAPROLITE IS ALSO AN EXAMPLE.	OF AN INTERVENING IMPERVIOUS STRATUM,  RESIDUAL (RES.) SOIL - SOIL FORMED IN PLACE BY THE WEATHERING OF ROCK.  ROCK QUALITY DESIGNATION (ROD) - A MEASURE OF ROCK QUALITY DESCRIBED BY TOTAL LENGTH OF ROCK SEGMENTS EQUAL TO OR GREATER THAN 4 INCHES DIVIDED BY THE TOTAL LENGTH OF CORE
HARD > 30 > 4  TEXTURE OR GRAIN SIZE	RECOMMENDATION SYMBOLS	ROCK HARDNESS	RUN AND EXPRESSED AS A PERCENTAGE.  SAPPOLITE (SAP.) - RESIDUAL SOIL THAT RETAINS THE RELIC STRUCTURE OR FABRIC OF THE PARENT
U.S. STD. SIEVE SIZE 4 10 40 60 200 270  DPENING (MM) 4.76 2.00 0.42 0.25 0.075 0.053	UNDERCUT  UNCLASSIFIED EXCAVATION - UNSUITABLE WASTE  UNCLASSIFIED EXCAVATION - ACCEPTABLE, BUT NOT TO BE UNCLASSIFIED EXCAVATION - UNSUITABLE DECRADABLE ROCK  UNDERCUT  UNDERCUT  UNCLASSIFIED EXCAVATION - UNSUITABLE BUT NOT TO BE UNCLASSIFIED EXCAVATION - EXCEPTABLE DECRADABLE ROCK  UNDERCUT  UNDERCUT  UNCLASSIFIED EXCAVATION - UNSUITABLE BUT NOT TO BE TO B	VERY HARD CANNOT BE SCRATCHED BY KNIFE OR SHARP PICK. BREAKING OF HAND SPECIMENS REQUIRES SEVERAL HARD BLOWS OF THE GEOLOGIST'S PICK.  HARD CAN BE SCRATCHED BY KNIFE OR PICK ONLY WITH DIFFICULTY. HARD HAMMER BLOWS REQUIRED TO DETACH HAND SPECIMEN.	ROCK, <u>SILL</u> - AN INTRUSIVE BODY OF IGNEOUS ROCK OF APPROXIMATELY UNIFORM THICKNESS AND RELATIVELY THIN COMPARED WITH ITS LATERAL EXTENT, THAT HAS BEEN EMPLACED PARALLEL TO THE BEDDING OR SCHISTOSITY OF THE INTRUDED ROCKS.
BOULDER COBBLE GRAVEL SAND SAND (CDL.)  (GR.) (CSE. SD.) (F SD.) (SL.)	ABBREVIATIONS	MODERATELY CAN BE SCRATCHED BY KNIFE OR PICK, GOUGES OR GROOVES TO 0.25 INCHES DEEP CAN BE EXCAVATED BY HARD BLOW OF A GEOLOGIST'S PICK, HAND SPECIMENS CAN BE DETACHED	SLICKENSIDE - POLISHED AND STRIATED SURFACE THAT RESULTS FROM FRICTION ALONG A FAULT OR SLIP PLANE.
GRAIN MM 305 75 2.0 0.25 0.05 0.005 SIZE IN. 12 3  SOIL MOISTURE - CORRELATION OF TERMS	AR - AUGER REFUSAL MED MEDIUM YST - VANE SHEAR TEST BT - BORING TERMINATED MICA MICACEOUS WEA WEATHERED  CL CLAY MOD MODERATELY 7 - UNIT WEIGHT CPT - CONE PENETRATION TEST NP - NON PLASTIC 7 - DIRY UNIT WEIGHT	BY MODERATE BLOWS.  MEDIUM CAN BE GROOVED OR GOUGED 0.05 INCHES DEEP BY FIRM PRESSURE OF KNIFE OR PICK POINT.  CAN BE EXCAVATED IN SMALL CHIPS TO PEICES 1 INCH MAXIMUM SIZE BY HARD BLOWS OF THE POINT OF A GEOLOGIST'S PICK.	STANDARD PEMETRATION TEST (PEMETRATION RESISTANCE) (SPT) - NUMBER OF BLOWS IN OR BPF) OF A 140 LB. HAMMER FALLING 30 INCHES REQUIRED TO PRODUCE A PEMETRATION OF 1 FOOT INTO SOIL WITH A 2 INCH OUTSIDE DIAMETER SPLIT SPOON SAMPLER. SPT REFUSAL IS PEMETRATION EQUAL TO OR LESS THAN 0.1 FOOT PER 60 BLOWS.
SOIL MOISTURE SCALE FIELD MOISTURE GUIDE FOR FIELD MOISTURE DESCRIPTION	CSE COARSE ORG ORGANIC  ORD - DILATOMETER TEST PMT - PRESSUREMETER TEST SAMPLE ABBREVIATIONS  DPT - DYNAMIC PENETRATION TEST SAP SAPROLITIC S - BULK	SOFT CAN BE GROVED OR GOUGED READILY BY KNIFE OR PICK. CAN BE EXCAVATED IN FRAGMENTS FROM CHIPS TO SEVERAL INCHES IN SIZE BY MODERATE BLOWS OF A PICK POINT. SMALL, THIN PICCES CAN BE BROKEN BY FINGER PRESSURE.	STRATA CORE RECOVERY (SREC.) - TOTAL LENGTH OF STRATA MATERIAL RECOVERED DIVIDED BY TOTAL LENGTH OF STRATUM AND EXPRESSED AS A PERCENTAGE.
- SATURATED - USUALLY LIQUID; VERY WET, USUALLY (SAT.) FROM BELOW THE GROUND WATER TABLE	e - VOID RATIO SD SAND, SANDY SS - SPLIT SPOON F - FINE SL SILT, SILTY ST - SHELBY TUBE FOSS, - FOSSILIFEROUS SLI SLIGHTLY RS - ROCK	VERY  CAN BE CARVED WITH KNIFE. CAN BE EXCAVATED READILY WITH POINT OF PICK, PIECES 1 INCH SOFT  OR MORE IN THICKNESS CAN BE BROKEN BY FINGER PRESSURE. CAN BE SCRATCHED READILY BY FINGERNAIL.	STRATA ROCK QUALITY DESIGNATION (SROD) - A MEASURE OF ROCK QUALITY DESCRIBED BY TOTAL LENGTH OF ROCK SEGMENTS WITHIN A STRATUM EQUAL TO OR GREATER THAN 4 INCHES DIVIDED BY THE TOTAL LENGTH OF STRATA AND EXPRESSED AS A PERCENTAGE.  TOPSOIL (TS.) - SURFACE SOILS USUALLY CONTAINING ORGANIC MATTER.
PLASTIC   SEMISOLID; REQUIRES DRYING TO ATTAIN OPTIMUM MOISTURE	FRAC FRACTURED, FRACTURES TCR - TRICONE REFUSAL RT - RECOMPACTED TRIAXIAL FRAGS FRAGMENTS	FRACTURE SPACING BEDDING	BENCH MARK: BM #2: RR SPIKE IN 18" SWEET GUM -L- STA. 15+II.8. 29.4' I
(PI) PL PLASTIC LIMIT ATTAIN OPTIMUM MOISTURE	HI HIGHLY V - VERY RATIO  EQUIPMENT USED ON SUBJECT PROJECT	TERM SPACING TERM THICKNESS VERY WIDE MORE THAN 10 FEET VERY THICKLY BEDDED 4 FEET	ELEVATION: 492.53 FEET
OM OPTIMUM MOISTURE - MOIST - (M) SOLID; AT OR NEAR OPTIMUM MOISTURE SL SHRINKAGE LIMIT	DRILL UNITS: ADVANCING TOOLS: HAMMER TYPE:	MIDE	NOTES:
- DRY - (D) REQUIRES ADDITIONAL WATER TO ATTAIN OPTIMUM MOISTURE	CME-45C CLAY BITS X AUTOMATIC MANUAL  6° CONTINUOUS FLIGHT AUGER CORE SIZE:  8° HOLLOW AUGERS	CLOSE 0.16 TO 1 FOOT VERY THINLY BEDDED 0.03 - 0.16 FEET VERY CLOSE LESS THAN 0.16 FEET THICKLY LAMINATED 0.008 - 0.03 FEET THINLY LAMINATED < 0.008 FEET  INDURATION	-
PLASTICITY  PLASTICITY INDEX (P) DRY STRENGTH		FOR SEDIMENTARY ROCKS, INDURATION IS THE HARDENING OF MATERIAL BY CEMENTING, HEAT, PRESSURE, ETC.	
NON PLASTIC 0-5 VERY LOW SLIGHTLY PLASTIC 6-15 SLIGHT	VANE SHEAR TEST UNGCARBIDE INSERTS	FRIABLE RUBBING WITH FINGER FREES NUMEROUS GRAINS: GENTLE BLOW BY HAMMER DISINTEGRATES SAMPLE.	
MODERATELY PLASTIC 16-25 MEDIUM HIGHLY PLASTIC 26 OR MORE HIGH  COLOR	XX CASING   X W ADVANCER   POST HOLE DIGGER	MODERATELY INDURATED  GRAINS CAN BE SEPARATED FROM SAMPLE WITH STEEL PROBE; BREAKS EASILY WHEN HIT WITH HAMMER.  GRAINS ARE DIFFICULT TO SEPARATE WITH STEEL PROBE;	
DESCRIPTIONS MAY INCLUDE COLOR OR COLOR COMBINATIONS (TAN, RED, YELLOW-BROWN, BLUE-GRAY).  MODIFIERS SUCH AS LIGHT, DARK, STREAKED, ETC. ARE USED TO DESCRIBE APPEARANCE.	CORE BIT SOUNDING ROO VANE SHEAR TEST	INDURATED DIFFICULT TO BREAK WITH HAMMER.  EVIDEMELY INDURATED SHARP HAMMER BLOWS REQUIRED TO BREAK SAMPLE;	
	<u> </u>	SAMPLE BREAKS ACROSS GRAINS.	DATE: 8-15-1

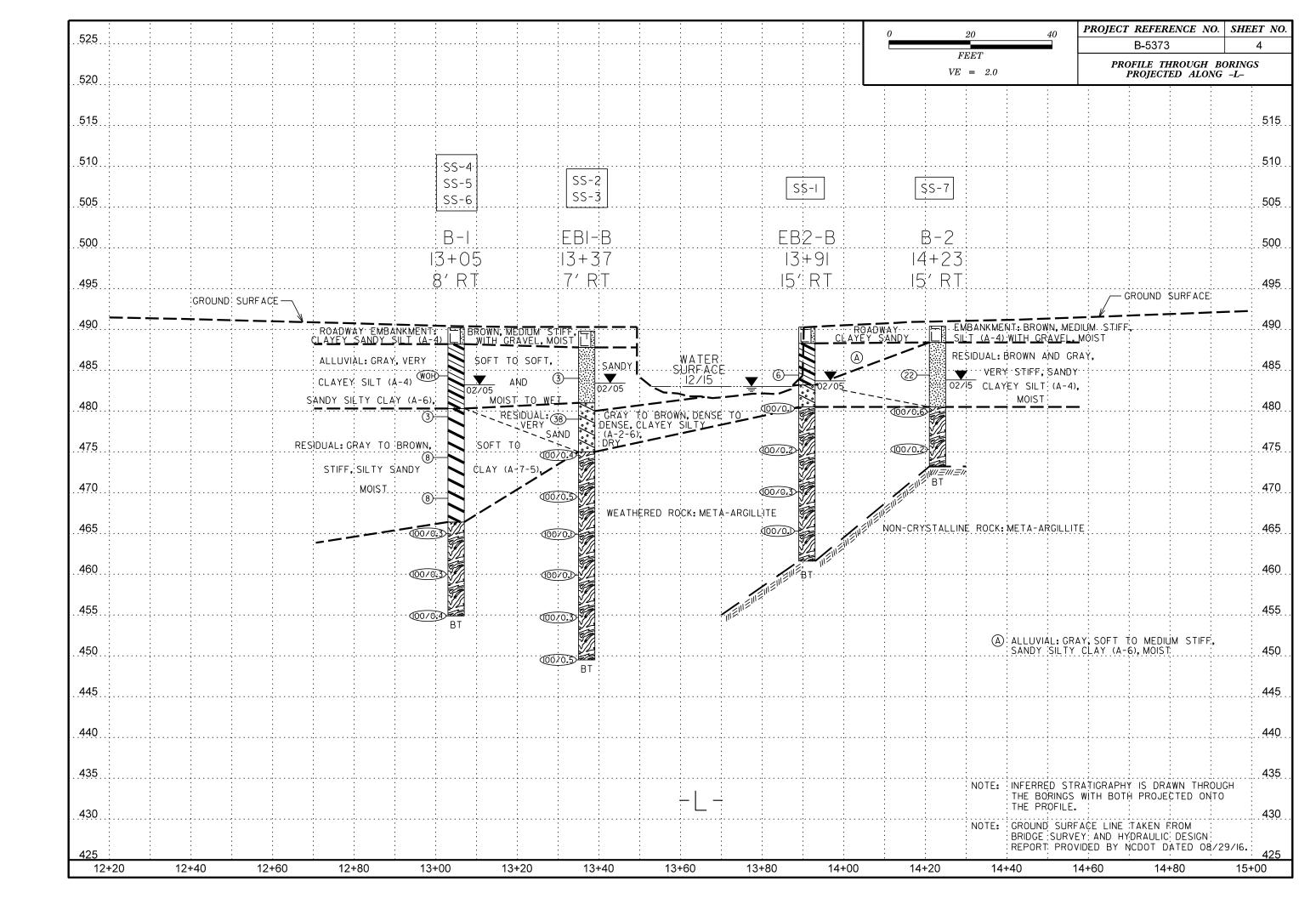
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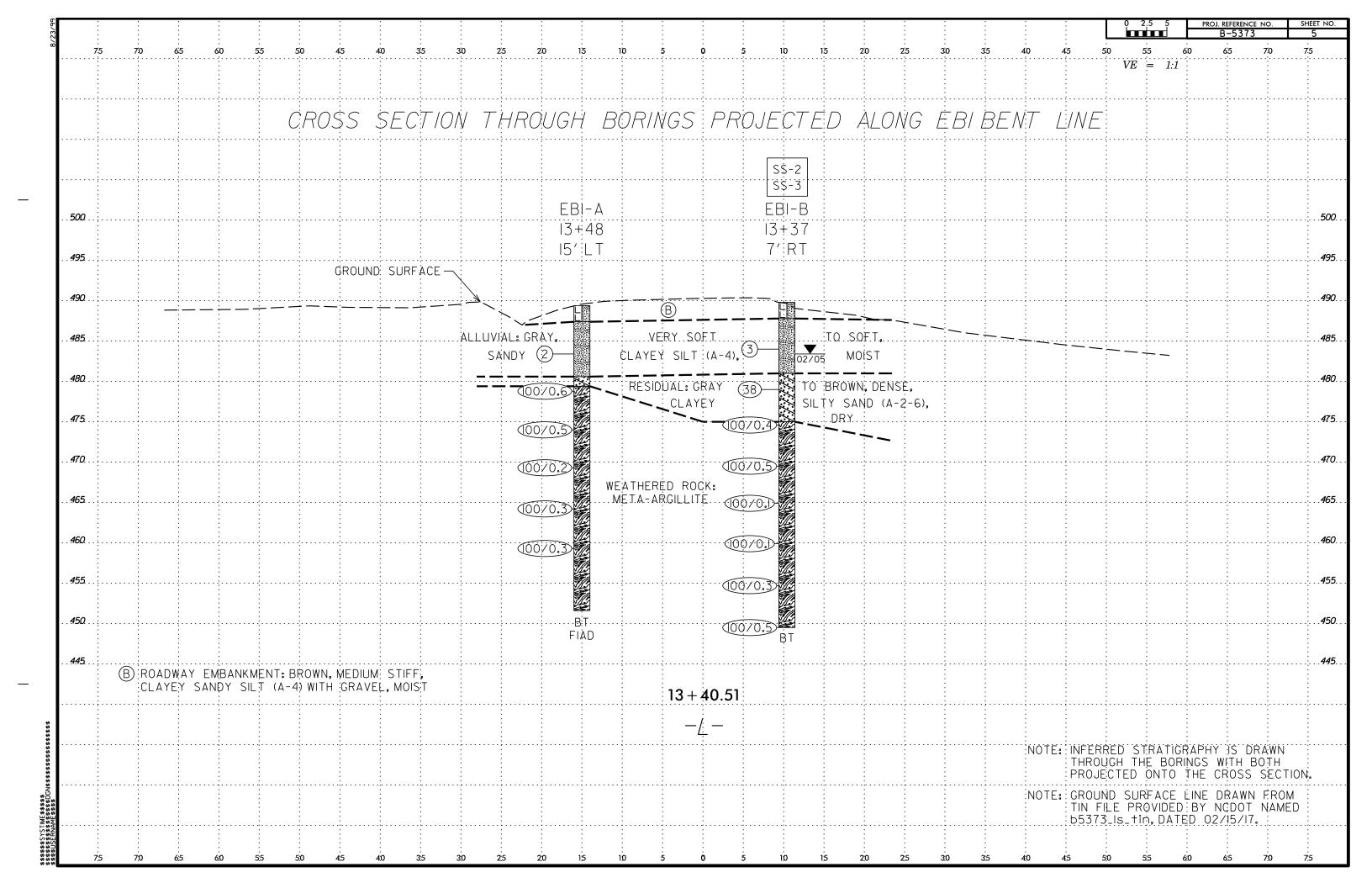
#### NORTH CAROLINA DEPARTMENT OF TRANSPORTATION DIVISION OF HIGHWAYS GEOTECHNICAL ENGINEERING UNIT

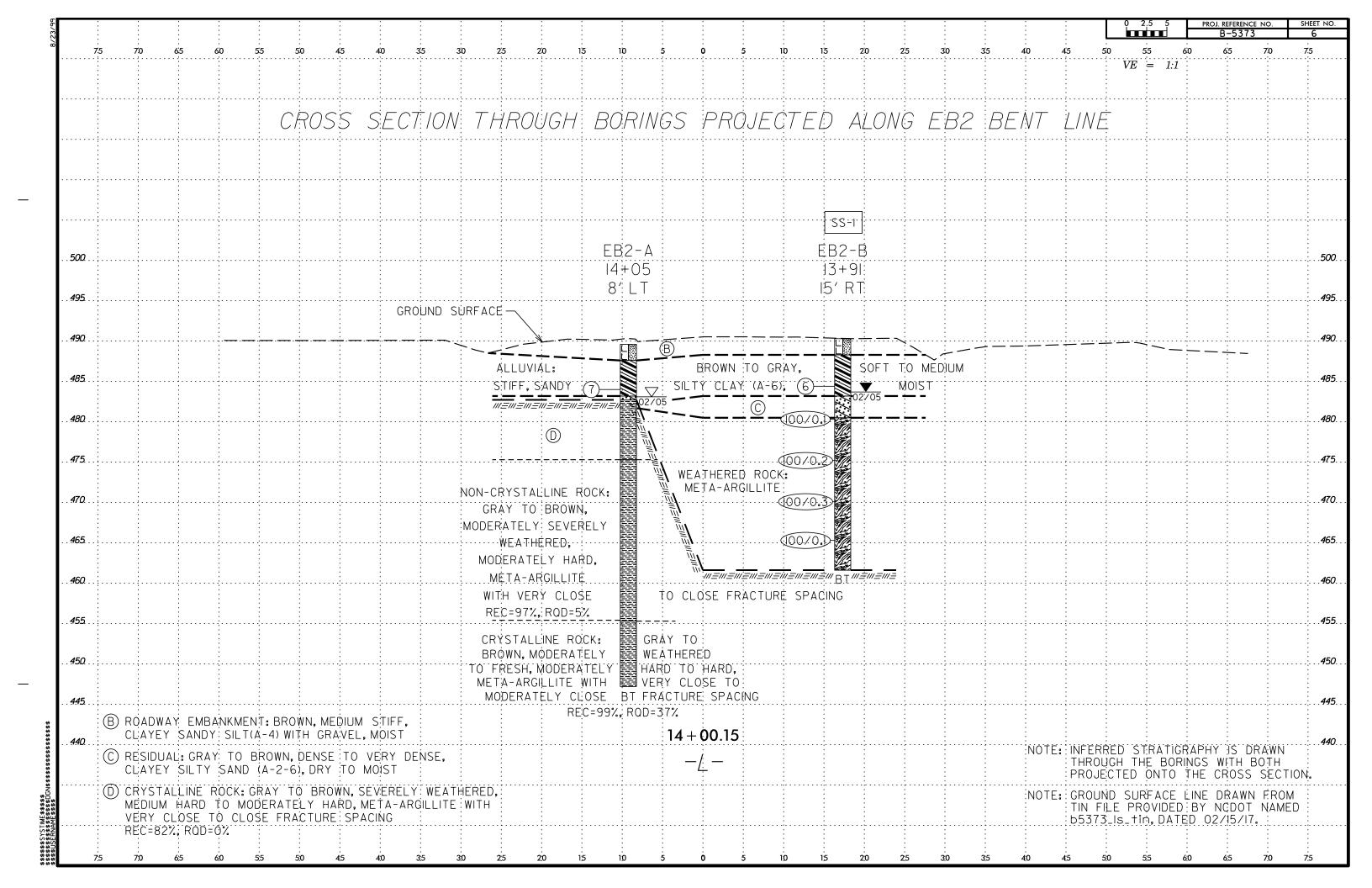
#### SUBSURFACE INVESTIGATION

FROM AASHTO LRFD BRIDGE DESIGN SPECIFICATIONS  AASHTO LRFD Figure 10.4.6.4-2 — Determination of GSI for Tectonically Deformed Heterogeneous Rock Masses (Marinos and Hoek, 2000)  AASHTO LRFD Figure 10.4.6.4-2 — Determination of GSI for Tectonically Deformed Heterogeneous Rock Masses (Marinos and Hoek, 2000)											
GEOLOGICAL STRENGTH INDEX (GSI) FOR JOINTED ROCKS (Hoek and Marinos, 2000)	S O O O	ъ		sec	8 0 0 0	GSI FOR HETEROGENEOUS ROCK MASSES SUCH AS FLYSCH (Marinos.P and Hoek E., 2000)					
From the lithology, structure and surface conditions of the discontinuities, estimate the average value of GSI. Do not try to be too precise. Quoting a range from 33 to 37 is more realistic than stating that GSI = 35. Note that the table does not apply to structurally controlled failures. Where weak planar structural planes are present in an unfavorable orientation with respect to the excavation face, these will dominate the rock mass behaviour. The shear strength of surfaces in rocks that are prone to deterioration as a result of changes in moisture content will be reduced if water is present. When working with rocks in the fair to very poor categories, a shift to the right may be made for wet conditions. Water pressure is dealt with by effective stress analysis.	SURFACE CONDITIONS VERY GOOD Very rough, fresh unweathered surface	GOOD Rough, slightly weathered, iron stained surfaces	rmin Smooth, moderately weathered and altered surfaces	POOR Slickensided, highly weathered surfaction with compact coatings or fillings or or angular fragments	VERY POOR Slickensided, highly weathered surfac with soft clay coatings or fillings	precise. Quoting a range from 33 to 37 is more realistic than giving GSI = 35. Note that the Hoek-Brown criterion does not apply to structurally controlled failures. Where unfavourably oriented continuous weak planar discontinuities are present, these will dominate the behaviour of the rock mass.  The strength of some rock masses is reduced by the presence of groundwater and this can be allowed for by a slight shift to the right in the columns for fair,	VERY POOR - Very smooth, slicken- sided or highly weathered surfaces				
STRUCTURE	DE	CREASING SURFA	ACE QUA		<b>⇒</b>	COMPOSITION AND STRUCTURE					
INTACT OR MASSIVE - intact rock specimens or massive in situ rock with few widely spaced discontinuities	90 80 -			, N/A	N/A	A. Thick bedded, very blocky sandstone The effect of pelitic coatings on the bedding planes is minimized by the confinement of the rock mass. In shallow tunnels or slopes these bedding planes may cause structurally controlled instability.					
BLOCKY - well interlocked undisturbed rock mass consisting of cubical blocks formed by three intersecting discontinuity sets	ROCK PIE	70 60				B. Sand- stone with stone and thin inter- layers of layers of layers and layers  B. Sand- stone and stone and thin inter- layers of laye					
VERY BLOCKY - interlocked, partially disturbed mass with multi-faceted angular blocks formed by 4 or more joint sets	OCKING OF	50				B. Sand- stone with stone and thin inter- layers of siltstone siltstone amounts  D. Siltstone or silty shale with sand- stone layers stone layers  B C D E					
BLOCKY/DISTURBED/SEAMY - folded with angular blocks formed by many intersecting discontinuity sets. Persistence of bedding planes or schistosity	ASING INTERL		40	30		C.D. E. and G - may be more or less folded than illustrated but this does not change the strength. Tectonic deformation, faulting and loss of continuity moves these categories to F and H.					
DISINTEGRATED - poorly inter- locked, heavily broken rock mass with mixture of angular and rounded rock pieces	DECREK			20		G. Undisturbed silty or clayey shale with or without a few very thin sandstone layers  H. Tectonically deformed silty or clayey shale forming a chaotic structure with pockets of clay. Thin layers of sandstone are transformed	10				
LAMINATED/SHEARED - Lack of blockiness due to close spacing of weak schistosity or shear planes	V N/A	N/A			10	→ Means deformation after tectonic disturbance	DATE: 8-19				

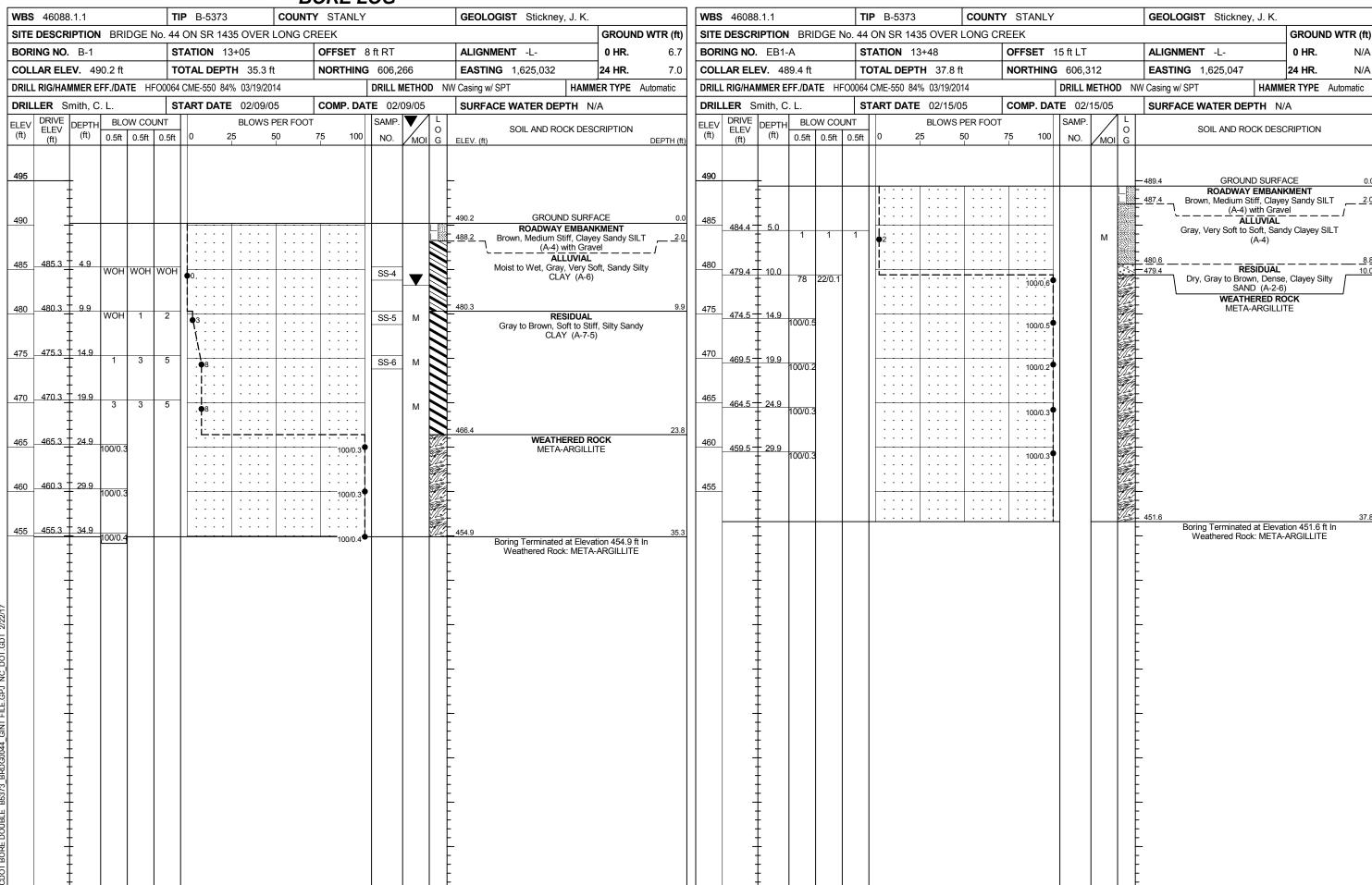








#### GEOTECHNICAL BORING REPORT BORE LOG



#### GEOTECHNICAL BORING REPORT BORE LOG

BORE LOG					
WBS         46088.1.1         TIP         B-5373         COUNTY         STANLY	GEOLOGIST Stickney, J. K.	<b>WBS</b> 46088.1.1	TIP B-5373 COUN	TY STANLY	GEOLOGIST Stickney, J. K.
SITE DESCRIPTION BRIDGE No. 44 ON SR 1435 OVER LONG CREEK	GROUND WTR (ft)	SITE DESCRIPTION BRIDGE No	o. 44 ON SR 1435 OVER LONG (	CREEK	GROUND WTR (ft)
BORING NO. EB1-B STATION 13+37 OFFSET 7 ft RT	<b>ALIGNMENT</b> -L- <b>0 HR.</b> 6.3	BORING NO. EB2-A	STATION 14+05	OFFSET 8 ft LT	ALIGNMENT -L- 0 HR. 6.5
COLLAR ELEV. 489.8 ft         TOTAL DEPTH 40.3 ft         NORTHING 606,288	<b>EASTING</b> 1,625,054 <b>24 HR.</b> 6.4	COLLAR ELEV. 489.6 ft	TOTAL DEPTH 42.4 ft	<b>NORTHING</b> 606,346	<b>EASTING</b> 1,625,094 <b>24 HR.</b> N/A
DRILL RIG/HAMMER EFF./DATE   HFO0064 CME-550 84% 03/19/2014   DRILL METHOD   NW	Casing w/ SPT HAMMER TYPE Automatic	DRILL RIG/HAMMER EFF./DATE HFO	00064 CME-550 84% 03/19/2014	DRILL METHOD	NW Casing W/SPT & Core HAMMER TYPE Automatic
	SURFACE WATER DEPTH N/A	DRILLER Smith, C. L.	<b>START DATE</b> 02/15/05	<b>COMP. DATE</b> 02/15/05	SURFACE WATER DEPTH N/A
ELEV (ft)   DRIVE   DEPTH   BLOW COUNT   BLOWS PER FOOT   SAMP.   U   C   C   C   C   C   C   C   C   C	SOIL AND ROCK DESCRIPTION  ELEV. (ft)  DEPTH (ft)	ELEV CHIP CHIP CHIP CHIP CHIP CHIP CHIP CHIP		75 100 NO. MOI G	
490	489.8 GROUND SURFACE 0.0  ROADWAY EMBANKMENT	490		1 188	489.6 GROUND SURFACE 0.0
485 485.0 4.8 WOH 1 2 \$\int_{0}^{1} \cdot	487.8 Brown, Medium Stiff, Clayey Sandy SILT 2.0  (A-4) with Gravel ALLUVIAL  Moist, Gray, Very Soft to Soft, Sandy Clayey  SILT (A-4)	485 485.0 4.6 WOH 2	5		ROADWAY EMBANKMENT  Brown, Medium Stiff, Clayey Sandy SILT2.0  (A-4) with Gravel
480 480.0 9.8 10 22 16	481.0	480			RESIDUAL Gray to Brown, Dense, Clayey Silty Sand (A-2-6) NON-CRYSTALLINE ROCK
475 475.0 14.8 100/0.4 M	SAND (A-2-6)  475.0  WEATHERED ROCK	475			Gray to Brown, Moderately Severly Weathered, Medium Hard to Moderately 475.3 Hard, META-ARGILLITE with Very Close to Close Fracture Spacing
470 470 0 198	META-ARGILLITE	470			Gray to Brown, Moderately Weathered, Moderately Hard, META-ARGILLITE with Very Close to Close Fracture Spacing
465 465.0 24.8 100/0.1 100/0.1 100/0.1 100/0.1		465			
460 460.0 29.8 100/0.1		460			
455 455.0 34.8		455			455.3 34.3
450 450.0 39.8	449.5 40.3	450		Service of the control of the contro	Gray to Brown, Moderately Weathered to Fresh, Moderately Hard to Hard, META-ARGILLITE with Very Close to Moderately Close Fracture Spacing
100/0.5	Boring Terminated at Elevation 449.5 ft In Weathered Rock: META-ARGILLITE				42.4  Boring Terminated at Elevation 447.2 ft In
NCDOT BORE DOUBLE B5573_BRDG0044_GINT FILE GFJ NC_DOT.GDT 2722/7					Boring Terminated at Elevation 447.2 ft In Non-Crystalline Rock: META-ARGILLITE

#### GEOTECHNICAL BORING REPORT CORE LOG

										<u> </u>	KE LUG	1	
WBS	46088	3.1.1			TIP	B-537	3	C	OUNT	Υ 5	TANLY	<b>GEOLOGIST</b> Stickney, J. K.	
SITE	DESCR	IPTION	I BRI	DGE No.	44 OI	SR 1	435 OVE	R LO	NG CI	REE	(		GROUND WTR (ft)
BOR	ING NO.	EB2-	A		STA	ΓΙΟΝ	14+05			OF	SET 8 ft LT	ALIGNMENT -L-	<b>0 HR.</b> 6.5
COLI	LAR ELI	<b>EV.</b> 48	39.6 ft		тот	AL DE	PTH 42	4 ft		NO	<b>RTHING</b> 606,346	<b>EASTING</b> 1,625,094	<b>24 HR</b> . N/A
DRILL	RIG/HA	MMER E	FF./DA	TE HFOO	064 CN	IE-550	84% 03/19	/2014		1	DRILL METHOD NV	/ Casing W/SPT & Core HAMI	MER TYPE Automatic
DRIL	LER S	mith, C	. L.		STAI	RT DA	<b>TE</b> 02/1	5/05		СО	MP. DATE 02/15/05	SURFACE WATER DEPTH	I/A
	E SIZE				_		<b>N</b> 35.5 f					l .	
ELEV	RUN	DEPTH	RUN	DRILL	REC.	JN RQD	SAMP.	STR REC.	ATA RQD	L	_		
(ft)	ELEV (ft)	(ft)	(ft)	RATE (Min/ft)	(ft) %	(ft) %	NO.	(ft) %	(ft) %	G	ELEV. (ft)	DESCRIPTION AND REMARKS	DEPTH (ft)
482.7												Begin Coring @ 6.9 ft	
	482.7 <sub>-</sub> 480.5 -	6.9 9.1	2.2		(2.0) 91%	(0.0) 0%		(6.1) 82%	(0.0) 0%	謹	482.7 Gray to Brown Mode	NON-CRYSTALLINE ROCK rately Severly Weathered, Medium Ha	6.9 and to Moderately
480	-100.0		5.2		(4.1)	(0.0)		0270	0,0		Hard, META-ARG	GILLITE with Very Close to Close Frac GSI = 20-30	ture Spacing
	-	Ł			79%	0%						001 - 20-30	
475	475.3	14.3				(2.5)					475.3		14.3
	-	F	5.0		(4.8) 96%	(0.5) 9%		(19.3) 97%	(1.0) 5%			rately Weathered, Moderately Hard, M Very Close to Close Fracture Spacing	
		Ī									•	GSI = 30-40	
470	470.3	19.3	5.0		(4.8)	(0.0)					· <del>-</del>		
		ļ			96%	0%				屋	•		
465	465.3	24.3									· ·		
100	-	†	5.0		(4.9) 98%	(0.5) 10%					<del>-</del> ·		
	:	‡									•		
460	460.3	29.3	5.0		(4.8)	(0.0)					<del>-</del>		
		‡	5.5		96%	0%							
455	455.3 <sup>-</sup>	34.3									455.3		34.3
455	- 100.0	57.5	5.0		(4.9)	(2.5)		(8.0)	(3.0)		<ul> <li>Gray to Brown, Mode</li> </ul>	erately Weathered to Fresh, Moderate	ly Hard to Hard,
		‡			98%	50%		99%	37%		. META-ARGILLITE V	vith Very Close to Moderately Close Fr GSI = 60-70	acture Spacing
450	450.3	39.3			(2.4)	(0.5)					<del>-</del>		
	447.0	<u> </u>	3.1		(3.1) 100%	(0.5) 15%							
	447.2	42.4									447.2 Boring Termina	ted at Elevation 447.2 ft In Non-Crysta	42.4 alline Rock:
	_	t									<del>-</del>	META-ARGILLITE	
	-	ł											
		£									<u>.                                    </u>		
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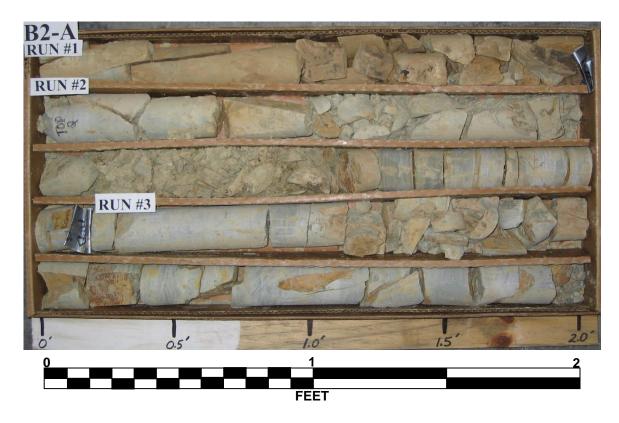
SHEET 9

#### CORE PHOTOGRAPH SHEET 10

WBS No. 46088.1.1 TIP No. B-5373

Site Description: Replace Bridge No. 44 over Long Creek on SR 1435

#### EB2-A





FEET



#### GEOTECHNICAL BORING REPORT BORE LOG

	E	BORE LOG											
<b>WBS</b> 46088.1.1		TY STANLY	GEOLOGIST Stickney, J. K.		<b>WBS</b> 4608			<b>TIP</b> B-5373	COUNTY ST	ANLY		GEOLOGIST Stickney,	
SITE DESCRIPTION BRIDGE NO		1	<u> </u>	GROUND WTR (ft)				44 ON SR 1435 OVER					GROUND WTR (ft)
BORING NO. EB2-B	STATION 13+91	OFFSET 15 ft RT	ALIGNMENT -L-	<b>0 HR.</b> 6.1	BORING NO			STATION 14+23		<b>SET</b> 15 ft F		ALIGNMENT -L-	<b>0 HR.</b> N/A
COLLAR ELEV. 490.3 ft	TOTAL DEPTH 28.7 ft	<b>NORTHING</b> 606,319	<b>EASTING</b> 1,625,099	<b>24 HR.</b> 6.6	COLLAR EL			TOTAL DEPTH 17.2		THING 606		<b>EASTING</b> 1,625,123	<b>24 HR.</b> 6.6
DRILL RIG/HAMMER EFF./DATE HFC		DRILL METHOD NV		IER TYPE Automatic				064 CME-550 84% 03/19/2					HAMMER TYPE Automatic
DRILLER Smith, C. L.	START DATE 02/08/05  NT BLOWS PER FOO	COMP. DATE 02/08/05  T SAMP. V L	SURFACE WATER DEPTH N	/A	DRILLER S		OW COUNT	START DATE 02/10/	PER FOOT	P. DATE 0		SURFACE WATER DEP	IH N/A
ELEV (ft) DEPTH (ft) 0.5ft 0.5ft 495		75 100 NO. MOI G		DEPTH (ft)	ELEV ELEV (ft)	12-:	t 0.5ft 0.5	<b>_</b>	50 75	100 NO	1/10	G  - - -	COURTERS.
480 480 4 9 9	5		488.3 GROUND SURF  ROADWAY EMBAN Brown, Medium Stiff, Clayt (A-4) with Grav ALLUVIAL Moist, Gray, Soft to Medium CLAY (A-6)  RESIDUAL Gray to Brown, Dense to Ver Silty SAND (A-5)	WMENT	485 485.4			2 • • 22		SS-	7	ROADWAY I  488.4 Brown, Medium Sti  (A-4) w  RES  Moist, Brown and C  Clayey:	O SURFACE 0.0  IMBANKMENT Iff, Clayey Sandy SILT 2.0  IDUAL IDUAL Iray, Very Stiff, Sandy  SILT (A-4)  9.9
475 475.4 14.9		100/0.11	WEATHERED R META-ARGILLI	OCK	475 475.5	<del>-</del>   37	63/0.1					META- <i>A</i>	RED ROCK RGILLITE  17.2  with Casing Advancer
470 470.4 19.9 100/0.3 465 465.4 24.9 100/0.1			. 461.6	28.7								Refusal at Elev	multicasing Advances ation 473.2 ft On ck: META-ARGILLITE
NCDOT BORE DOUBLE B5373_BKUG0044_GINI FILE.GFV NC_DVI.GDI (2021)			Boring Terminated with Ca Refusal at Elevation 4 Non-Crystalline Rock: MET	61.6 ft On									

### NORTH CAROLINA DEPARTMENT OF TRANSPORTATION DIVISION OF HIGHWAY MATERIALS & TESTS UNIT

MATERIALS & TESTS UNI
SOILS LABORATORY

1. I. P. No.	D-33/3	_				
	REPORT ON SAM	IPLES OF	SOILS FOR	QUAL	ITY	
Project	46088.1.1	County	STANLY		_ Owner	
Date: Sampled	1/5/05	Received	2/15/05		Reported	2/17/2005
Sampled from	BRIDGE			By	J E BEVE	RLY
Submitted by	N WAINAINA				1995	Standard Specifications
			_			-
720125 TO 720	131					
2/20/17						

#### TEST RESULTS

Proj. Sample No.	SS-1	SS-2	SS-3	SS-4	SS-5	SS-6
Lab. Sample No.	720125	720126	720127	720128	720129	720130
Retained #4 Sieve %	12	8	19	-	-	-
Passing #10 Sieve %	83	79	58	100	100	98
Passing #40 Sieve %	79	72	47	100	83	68
Passing #200 Sieve %	66	63	35	88	53	37

#### MINUS NO. 10 FRACTION

SOIL MORTAR - 100%							
Coarse Sand Ret - #60	%	6.1	10.5	25.1	1.0	28.9	43.7
Fine Sand Ret - #270	%	18.4	13.4	19.6	17.2	23.5	23.5
Silt 0.05 - 0.005 mm	%	31.0	37.7	31.0	35.2	35.4	24.7
Clay < 0.005 mm	%	44.5	38.5	24.3	46.6	12.1	8.1
Passing #40 Sieve	%	-	-	-	•	-	-
LOCATION	%	EB2-B	EB1-B	EB1-B	B-1	B-1	B-1

L. L.	34	30	34	35	55	48
P. I.	11	7	11	13	13	12
AASHTO Classification	A-6(6)	A-4(3)	A-2-6(0)	A-6(11)	A-7-5(6)	A-7-5(1)
Station	10+41	9+87	9+87	9+53	9+53	9+53
OFFSET	15 RT	9 RT	9 RT	10.5 RT	10.5 RT	10.5 RT
ALIGNMENT	L	L	L	L	L	L
Depth (Ft)	5.40	5.30	10.30	5.40	10.40	15.40
to	6.40	6.30	11.30	6.40	11.40	16.40

Soils Engineer

cc: J E BEVERLY
Soils File

M & T Form 503

SHEET 12

# NORTH CAROLINA DEPARTMENT OF TRANSPORTATION DIVISION OF HIGHWAY MATERIALS & TESTS UNIT SOILS LABORATORY

T. I. P. No.	B-5373	_						
	REPORT ON SAMPLES OF		SOILS FOR QUALITY					
Project	46088.1.1	County	STANLY 2/15/05		Owner			
Date: Sampled	1/5/05	Received			Reported	2/17/2005		
Sampled from	•			By	J E BEVERLY			
-	N WAINAINA				1995 Standard Specification		cations	
Submitted by	11 WIHWHUI				1773	Standard Speems	cations	
720125 TO 720 2/20/17	131	TE	ST RESULT	ΓS				
Proj. Sample N	0.	SS-7						
Lab. Sample N		720137						
Retained #4 S	ieve %	-						
Passing #10 Sieve %		93						
Passing #40 Sieve %		88						
Passing #200 S	Sieve %	67						
		MINUS	NO. 10 FRA	CTION				
SOIL MORTA								
Coarse Sand		9.9						
Fine Sand Ro		22.3						
Silt 0.05 - 0.		27.3						
Clay < 0.005		40.5						
Passing #40 S LOCATION	ieve %	B-2			1			
LOCATION	/0	D-2	<u> </u>					
L. L.		29						
P. I.		10						
AASHTO Clas	sification	A-4(5)						
Station		10+73						
OFFSET		14.5 RT						
ALIGNMENT		L						
Depth (Ft)		5.50						
	to	6.50						

Soils Engineer